(1/S001) STRUCTURAL DESIGN CRITERIA AND GENERAL SPECIFICATIONS

1. BUILDING CODE:
   A. IBC 2006/ASCE 7-05
   B. PRE ENGINEERED METAL BUILDING COLUMN BASE REACTIONS WERE PROVIDED BY LITTLE DIXIE CONSTRUCTION COMPANY (AMERICAN BUILDINGS, PROJECT NUMBER 61395501, DRAWING DATED 10-5-98). THIS BUILDING WAS DESIGNED UNDER THE BOCA 1996 CODE - SEE METAL BUILDING DRAWINGS.
   C. ALLSTATE CONSULTANTS LLC IS THE FOUNDATION SYSTEM SPECIALTY STRUCTURAL ENGINEER FOR THIS PROJECT ONLY. WE ARE NOT THE STRUCTURAL ENGINEER OF RECORD (SER) FOR THIS PROJECT.
   D. FOUNDATION DESIGN (SEE GEOTECHNICAL REPORT).
      1) MAX NET SOIL BEARING = 2000 PCF
      2) ACTIVE LATERAL EARTH PRESSURE = 40 PCF (GRAVEL BACKFILL PER GEOTECH, 45° FROM VERTICAL).

2. GEOTECHNICAL DATA AND FOUNDATION SYSTEM:
   E. THE FOLLOWING DESIGN INFORMATION IS FROM THE GEOTECHNICAL REPORT BY ALLSTATE CONSULTANTS LLC (PN: 08052.01) DATED 5-15-2008.
   A. FOOTINGS SHALL BEAR ON THE EXISTING STIFF NATIVE CLAY OR EXISTING STIFF CLAY DIRECTED BY A GEOTECHNICAL ENGINEER.
   B. FOOTINGS BEARING ON NATIVE UNDISTURBED SOIL OR EXISTING FILL ARE DESIGNED FOR A MAXIMUM NET BEARING CAPACITY OF 2,000 PSF
   C. SEE THE GEOTECHNICAL ENGINEERING REPORT BY ALLSTATE CONSULTANTS LLC FOR REMOVAL AND REPLACEMENT OR CONDITIONING OF SOIL BELOW AND DIRECTLY ADJACENT TO THE BUILDING FOUNDATION IF REQUIRED.
   D. ALL TOPSOIL AND OTHER UNSUITABLE BEARING MATERIAL SHALL BE REMOVED. A GEOTECHNICAL ENGINEER SHALL INSPECT THE EXCAVATED AREA TO ENSURE ALL MATERIALS REQUIRING REMOVAL HAVE BEEN REMOVED, ALL NECESSARY SURFACE COMPACTION IS PERFORMED, AND TO VERIFY THE SOIL BEARING CAPACITY USED FOR DESIGN.
   E. FROST COVER FROM EXTERIOR GRADE TO BOTTOM OF FOOTING SHALL NOT BE LESS THAN 3'-0". FOUNDATION SYSTEMS MAY REQUIRE LOWERING TO PROVIDE FROST DEPTH.
   F. BACKFILL AGAINST FOOTING STEM/BASEMENT WALLS INSIDE THE BUILDING SHALL BE MADE WITH COMPACTED CRUSHED STONE CONFORMING TO ASTM C33, SIZE 56, OR EQ.
   G. ALL BUILDING AND SITE WALLS SHALL BE BACKFILLED OUTSIDE THE STRUCTURE/BACKFILL SIDE USING A MINIMUM OF 24" OF CLEAN CRUSHED STONE AGAINST THE WALL WITH A FILTER CLOTH AROUND THE GRAVEL THAT SURROUNDS THE DRAIN TILE AT THE FOUNDATION WALL (ABOUT 24" SQ). GRANULAR BACKFILL ZONE SHALL EXTEND 45° OFF VERTICAL FROM THE BASE OF WALLS UNO.
   H. THE FOUNDATION SYSTEM IS DESIGNED FOR THE LOCAL PARAMETERS CONTAINED HEREIN. THIS DESIGN DOES NOT CONSIDER OR ACCOUNT FOR THE GLOBAL STABILITY OF THE SITE OR SLOPES.
   I. SITE GRADING SHALL BE CONFIGURED TO ALLOW ADEQUATE DRAINAGE AWAY FROM THE STRUCTURE. SEE SITE GRADING PLAN FOR THIS INFORMATION. UNLESS MORE DETAILED INFORMATION IS PROVIDED ON THE PLANS SLOPE EXTERIOR GRADE AWAY FROM THE STRUCTURE A MINIMUM OF 6" IN THE FIRST 10' UNO.

3. REINFORCED CONCRETE:
   A. DESIGN CODES: BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318), LATEST ADOPTION.
   B. MATERIAL STRENGTHS
      1) CONCRETE COMPRESSIVE STRENGTH (f'c) AT 28 DAYS:
         a. FOOTINGS 3000 PSI
         b. FOUNDATION WALLS 4000 PSI
         c. ALL OTHER CONCRETE 4000 PSI
      2) SLUMP
         a. WHEN WATER REDUCING ADMIXTURE IS ADDED TO INCREASE CONCRETE SUMP, CONCRETE SHALL HAVE A SLUMP OF 2" TO 4" BEFORE THE ADMIXTURE IS ADDED AND 8" (±1") AFTER.
         b. SLUMP WITHOUT WATER REDUCING ADMIXTURE
            1) FOUNDATION WALLS, FOOTINGS, AND SLABS-ON-GRADE 1" TO 3"
            2) ALL OTHER CONCRETE 5"
      3) ANCHOR RODS ASTM A307 AND THREADED RODS ASTM A36 (OR ASTM F1554, GRADE 36).
      4) REINFORCING STEEL
         a. BARS AND TIES ASTM A615, GRADE 60 (Fy=60ksi)
   C. NOTES:
      1) PLACEMENT OF CONCRETE AND REINFORCEMENT SHALL BE IN ACCORDANCE WITH ACI AND CRSI STANDARDS. LAP OR DOWEL CONTINUOUS BARS 48 BAR DIAMETERS TYPICAL UNLESS NOTED. NO WELDING OF REINFORCING IS PERMITTED.
      2) EXPOSED CONC SURFACES SHALL BE FINISHED TO A UNIFORM APPEARANCE.
      3) REINF SHOWN ON DETAILS IS FOR CONCEPT ONLY, FOLLOW CRSI PLACMENT AND COVER REQMTS UNO.
      4) FURNISH THE FOLLOWING CONCRETE COVER AT REINFORCING BARS UNLESS SHOWN OTHERWISE ON THE DRAWINGS:
         a. SLABS-ON-GRADE PLACE REINFORCING IN CENTER OF SLAB
         b. PEDESTALS 2" COVER AT TIES
         c. FOOTINGS/GRADE BEAMS 3" COVER AT BOTTOM AND SIDES
         d. WALLS 2" COVER WHERE EXPOSED TO SOIL OR WEATHER
      5) MAXIMUM AGGREGATE SIZE SHALL NOT EXCEED 3/4 THE CONCRETE COVER (I.E. 3/4" MAX AGGREGATE FOR 1" COVER)
      6) AT CORNERS OF ALL WALLS, FOOTINGS, GRADE BEAMS, ETC., SUPPLY CORNER BARS 4'-0" LONG (2'-0" EACH DIRECTION) MATCHING SIZE AND SPACING OF ALL HORIZONTAL BARS.
      7) ALL CONCRETE EXPOSED TO WEATHER SHALL BE AIR ENTRAINED PER ACI REQUIREMENTS FOR MODERATE EXPOSURE (5% FOR 3/4" MAX AGGREGATE).
      8) TESTING:
         a. ALL FIELD TESTING SHALL BE PERFORMED BY A CERTIFIED ACI FIELD TECHNICIAN.
         b. TAKE NOT LESS THAN THREE CYLINDERS FOR EACH CLASS OF CONCRETE, FOR EACH 150 CUBIC YARDS OR FRACTION THEREOF, FOR EACH DAY CONCRETE IS CAST, OR NOT LESS THAN ONCE FOR EACH 5,000 SQ FT OF SLAB OR WALL AREA.
         c. OF EACH SET OF THREE CYLINDERS, ONE SHALL BE TESTED AT 7 DAYS AND TWO AT 28 DAYS.

4. PRE-ENGINEERED METAL BUILDING:
   A. DESIGN CODE: SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS (AISC), AND METAL BUILDING MFG ASSOC (MBMA), LATEST ADOPTION OF THE METAL BUILDING SYSTEMS MANUAL.
   B. NOTES:
      1) CONTRACTOR SHALL INSPECT ALL MEMBERS WHILE DISMANTILING THE EXISTING METAL BUILDING TO ENSURE THAT ALL MEMBERS ARE NOT DAMAGED OR OUT OF PLUMB (REFERENCE THE AISC CODE OF STANDARD PRACTICE TOLERANCES). THE CONTRACTOR SHALL ALSO INSPECT ALL MEMBERS WHILE REASSEMBLING THE METAL BUILDING TO ENSURE NO DAMAGE OCCURRED DURING TRANSPORT. IF DAMAGE IS FOUND CONTACT THE ENGINEER IMMEDIATELY.
      2) ALL METAL BUILDING STRUCTURAL BOLTS SHALL BE REPLACED WITH NEW ASTM A325 OR A490 BOLTS WITH SIZE AND GRADE MATCHING THE METAL BUILDING REQUIREMENTS.

5. GENERAL NOTES:
   A. THE NOTES ON THE STRUCTURAL DRAWINGS ARE APPLICABLE TO ALL DRAWINGS IN ADDITION TO THE PROJECT SPECIFICATIONS. THESE NOTES TAKE PRECEDENCE OVER THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.
   B. DRAWINGS PROVIDE A SYSTEM FOR THE IN-PLACE STRUCTURE. METHODS OF CONSTRUCTION TO ACHIEVE THE IN-PLACE SYSTEM ARE NOT ADDRESSED UNLESS SPECIFICALLY NOTED.
   C. THE CONTRACTOR SHALL VERIFY THAT FOUNDATION SYSTEMS ADJACENT TO EXISITNG BUILDING/STRUCTURE FOUNDATIONS DO NOT UNDERMINE THE EXISTING FOUNDATION OR DAMAGE THE EXISTING STRUCTURE. UNDERPINNING, SOIL STABILIZATION, OR SHORING MAY BE REQUIRED DEPENDING ON FIELD CONDITIONS. THE DESIGN SHOWN HEREIN IS BASED ON ASSUMPTIONS FROM EXISTING PLANS AND WAS NOT FIELD VERIFIED.
   D. DURING ERECTION OF THE BUILDING, THE CONTRACTOR SHALL BE RESPONSIBLE FOR TEMPORARY BRACING TO WITHSTAND ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING LATERAL LOADS, STOCKPILES OF MATERIAL AND EQUIPMENT. SUCH BRACING SHALL BE LEFT IN PLACE AS LONG AS REQUIRED FOR SAFETY AND UNTIL ALL FRAMING IS IN PLACE.
   E. UNLESS NOTED, SUBMIT SHOP DRAWINGS OF ALL FABRICATED MATERIALS FOR REVIEW. DESIGN DRAWINGS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS WITHOUT WRITTEN CONSENT OF ENGINEER. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY WERE CHECKED, BEAR THE INITIAL OF THE CHECKER AND ARE STAMPED "APPROVED" BY THE GENERAL CONTRACTOR. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR BUT NOT LIMITED TO THE FOLLOWING:
      1) CONCRETE MIX DESIGNS.
      2) REINFORCING STEEL IN CONCRETE SHOWING WALL ELEVATIONS AND CONTROL JOINTS TYP.
      3) PRODUCT SPECIFICATIONS FOR ENGINEERED SYSTEMS AND ACCESSORIES INCLUDING WOOD, STEEL, CONCRETE, AND MASONRY SYSTEMS.
      4) MATERIAL CERTIFICATES FOR STRUCTURAL MATERIALS INCLUDING WOOD, STEEL, CONCRETE, AND MASONRY.
   F. WHERE A DETAIL, TYPICAL DETAIL, SECTION, TYPICAL SECTION, OR A NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL THE LIKE OR SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
   G. ALL DIMENSIONS SHOWN ON THESE DRAWINGS SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO THE APPROVAL OF SHOP DRAWINGS BY THE ENGINEER. CONFLICTS ON THE STRUCTURAL, ARCHITECTURAL, MEP, CIVIL, OR OTHER DRAWINGS SHALL BE COMMUNICATED TO THE ENGINEER PRIOR TO COMPLETING ANY WORK.
   H. ALL DIMENSIONS SHOWN ON THESE DRAWINGS SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO THE APPROVAL OF SHOP DRAWINGS BY THE ENGINEER. ALL OF THE PLANS DEPICTING EXISTING ADJACENT STRUCTURES WERE CREATED FROM ASSUMPTIONS AND APPROXIMATE DIMENSIONS. THEREFORE, CONFIRMATION OF ALL DIMENSIONS IS ESSENTIAL TO THE PROPER FIT OF FABRICATED ITEMS.

6. SPECIAL INSPECTION REQUIREMENTS:
   A. APPROVED FABRICATORS
      1) WORK PERFORMED AT A FACILITY/PLANT SHALL BE APPROVED BY A NATIONALLY CERTIFIED ORGANIZATION.
      2) A COPY OF CURRENT CERTIFICATION SHALL BE SUBMITTED.
   B. SPECIAL INSPECTION AGENCIES
      1) AGENCIES SHALL BE UNDER DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MISSOURI.
      2) LABORATORY FACILITIES SHALL MEET APPLICABLE ASTM/ICC SPECIFICATIONS.
      3) EXCEPT FOR REGISTERED PROFESSIONAL ENGINEERS, ALL TESTING TECHNICIANS, INSPECTORS, AND ENGINEERS PERFORMING SPECIAL INSPECTIONS SHALL BE CERTIFIED PER BUILDING CODE REQUIREMENTS AS APPLICABLE FOR THE ITEM TESTED.
      4) A FINAL SPECIAL INSPECTION REPORT SHALL BE SUBMITTED.
   C. REINFORCED CONCRETE
      1) PERIODIC INSPECTION OF PLACEMENT OF CONCRETE.
      2) CONCRETE STRENGTH EVALUATION IN ACCORDANCE WITH NOTES AND ACI 318.
      3) ANCHOR BOLTS, VERIFY ANCHOR BOLT DIAMETER, LOCATION, AND EMBEDMENT LENGTH.
      4) VERIFY PLACEMENT OF REINFORCING STEEL FOR PROPER SIZE, GRADE, SPACING, CLEARANCES, SPLICE LENGTHS, AND COVER FOR CONFORMANCE WITH APPROVED PLANS AND SPECIFICATIONS.
   D. SOILS, EXCAVATION, FILLING, DRILLED PIERS, AND RETAINING WALLS
      1) VERIFY BEARING MATERIAL.
      2) VERIFY ENGINEERED FILL IS PLACED IN ACCORDANCE WITH GEOTECHNICAL ENGINEER'S REQUIREMENTS.
      3) VERIFY SIZE AND DEPTH OF FOOTINGS.

(2/S001) CONCRETE FOOTING, PEDESTAL, AND STEEL COLUMN SCHEDULE

MARK	FOOTING SIZE	FOOTING THK	FOOTING REINF	FOOTING TOP EL	PEDESTAL SIZE	PEDESTAL TOP EL	ANCHOR BOLTS	MTL BLDG REACTIONS (KIPS) NET VERT (UP)	VERT (DN)	NET HORZ (IN)	HORZ (OUT)	REMARKS	MARK
A-1	4'-0"x4'-0"	12"	(5) #5's EW, BOT	94'-0"	24"x24"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		A-1
A-2	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		A-2
A-3	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		A-3
A-4	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		A-4
A-5	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		A-5
A-6	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		A-6
A-7	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		A-7
A-8	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	24"x24"	100'-0"	(2) 3/4"ø	15.0	46.7	61.7	61.7		A-8
B-1	4'-0"x4'-0"	12"	(5) #5's EW, BOT	94'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		B-1
B-8	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		B-8
C-1	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		C-1
C-8	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		C-8
D-1	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		D-1
D-8	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		D-8
E-1	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		E-1
E-8	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		E-8
F-1	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		F-1
F-8	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		F-8
G-1	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		G-1
G-8	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	18"x18"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		G-8
H-1	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	24"x24"	100'-0"	(2) 3/4"ø	15.0	46.7	3.0	3.0		H-1
H-2	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		H-2
H-3	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		H-3
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H-5	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		H-5
H-6	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		H-6
H-7	7'-0"x12'-0"	18"	#4's AT 18"o.c. EW TOP AND #6's AT 9"o.c. EW BOT	97'-0"	24"x24"	100'-0"	(6) 1 1/4"ø	15.0	46.7	61.7	61.7		H-7
H-8	4'-0"x4'-0"	12"	(5) #5's EW, BOT	97'-0"	24"x24"	100'-0"	(2) 3/4"ø	4.3	7.9	3.0	3.0		H-8

NOTES:
1. TYPICAL ANCHOR RODS SEE DETAIL 9/S501.
2. TYPICAL STEEL COLUMN BASE PLATES AND ANCHOR RODS LAYOUT SEE MTL BLDG MFR DWGS.
3. TYPICAL CONCRETE PEDESTAL REINFORCING SEE DETAIL 7/S501.
4. ANCHOR RODS SHALL PENETRATE BASE PLATE W/ OVERSIZED HOLES UNLESS NOTED. PROVIDE ASTM F436 WASHERS PER AISC REQMTS BETWEEN THE PLATE AND TOP NUT TYPICAL UNLESS NOTED.
5. FOOTINGS AND PEDESTALS ARE CENTERED UNDER COLUMNS UNLESS NOTED OR SHOWN ON PLANS.
6. PROVIDE (3) #3 TIES AT 3"o.c. AT TOP OF ALL PEDESTALS UNO BEGINNING 2" FROM TOP THEN SPACING INDICATED ON DETS. ALSO, PROVIDE ONE (1) TIE 2" FROM BOT OF ALL PEDESTALS UNO.
7. PROVIDE PEDESTAL DWLS WITH STD ACI TENSION HOOKS INTO FTG (LAP IN PEDESTAL PER S001 NOTES) BOT TO MATCH VERT PEDESTAL STEEL OR HOOK VERT PEDESTAL STEEL INTO FOOTINGS WITH ACI TENSION HOOKS INTO FTG BOT TYP UNO.
8. PLACE CONC WALL PEDESTALS/COLUMNS MONOLITHICALLY WITH WALL UNO.
9. MTL BLDG (PRE-ENGINEERED BLDG) REACTIONS WERE PROVIDED BY AMERICAN BUILDINGS.

TYPICAL STRUCTURAL ABBREVIATIONS AND SYMBOLS

ACI	AMERICAN CONCRETE INSTITUTE	EW	EACH WAY	PL	PLATE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	EXIST	EXISTING	plf	POUNDS PER FOOT
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS	EXP	EXPANSION	PROJ	PROJECTION
AB	ANCHOR BOLT	EXT	EXTERIOR	psf	POUNDS PER SQUARE FOOT
ADJ	ADJACENT	FIN	FINISH	psi	POUNDS PER SQUARE INCH
AFF	ABOVE FINISHED FLOOR	FD	FLOOR DRAIN	R	RADIUS
AHU	AIR HANDLING UNIT	FLG	FLANGE	RD	ROOF DRAIN
ALT	ALTERNATE	FLR	FLOOR	RE	REFERENCE
L	ANGLE	FND	FOUNDATION	REINF	REINFORCEMENT
ARCH	ARCHITECT	F.O.	FACE OF	REQD	REQUIRED
BFF	BELOW FINISHED FLOOR	FTG	FOOTING	REV	REVISION
BLDG	BUILDING	GA	GAGE	RTU	ROOF TOP UNIT
BLKG	BLOCKING	GALV	GALVANIZED	SAH	STRONG AXIS HORIZONTAL
BM	BEAM	GDR	GIRDER	SCHED	SCHEDULE
BOC	BOTTOM OF CAISSON	GYP	GYPSUM	SECT	SECTION
BOT	BOTTOM	H.A.S.	HEADED ANCHOR STUD	SHT	SHEET
BRG	BEARING	HK	HOOK	SIM	SIMILAR
BTWN	BETWEEN	HOR	HORIZONTAL	SJI	STEEL JOIST INSTITUTE
C	COMPRESSION	INCL	INCLUDING	SPA	SPACING
CL	CENTER LINE	INSUL	INSULATION	SPECS	SPECIFICATIONS
CJ	CONTROL/CONST JOINT	INT	INTERIOR	STD	STANDARD
CLR	CLEAR	JST	JOIST	STIFF	STIFFENER
COL	COLUMN	JT	JOINT	STL	STEEL
CONC	CONCRETE	K	KIP = 1000 LBS	SQ	SQUARE
CONN	CONNECTION	K-FT	KIP-FEET (Moment)	SYM	SYMMETRICAL
CONT	CONTINUOUS	LOC	LOCATION	T	TENSION
DBA	DEFORMED BAR ANCHOR	LLH	LONG LEG HORIZONTAL	T&B	TOP AND BOTTOM
DIA or ø	DIAMETER	LLV	LONG LEG VERTICAL	THK	THICK OR THICKNESS
DIM	DIMENSION	MAX	MAXIMUM	T.O.	TOP OF
DET	DETAIL	MECH	MECHANICAL	TOBL	TOP OF BRICK LEDGE
DN	DOWN	MIN	MINIMUM	TOC	TOP OF CONCRETE
DWG	DRAWING	MISC	MISCELLANEOUS	TOF	TOP OF FOOTING
DWL	DOWEL	MFR	MANUFACTURER	TOM	TOP OF MASONRY
EA	EACH	MO	MASONRY OPENING	TOS/TOB	TOP OF STEEL/BEAM
EE	EXTENDED ENDS	MTL	METAL	TOW	TOP OF WALL
EF	EACH FACE	#	NUMBER OR POUND	TRS	TRUSS
EL	ELEVATION	NIC	NOT IN CONTRACT	TYP	TYPICAL
EMBED	EMBEDMENT LENGTH	NTS	NOT TO SCALE	UNO	UNLESS NOTED OTHERWISE
EQ	EQUAL/EQUIVALENT	o.c.	ON CENTER	VERT	VERTICAL
		OPNG	OPENING	W/	WITH
		OPP	OPPOSITE HAND	WP	WORKING POINT
		P.C.	PRECAST CONCRETE	WWF	WELDED WIRE FABRIC

DET NUMBER (TOP) / SHT NUMBER (BOT)

COL GRID LINE

CONC GRADE BM/WALL

COL/FTG/PED

KEYNOTE

MK-DIA / EL — CONC DRILLED PIER DIA AND TOP EL

SPOT EL

EL MK

N — PLAN NORTH OR REFERENCE NORTH

W12x19 (6) c=1/2" TOB EL 100'-0" [+1 1/2"]
a. b. c. d. e.

a. DESIGNATION OF STRUCT MEMBER
b. NUMBER OF EQUALLY SPACED 3/4"ø x4" LG (3" LG AT AEROBICS AREA) SHEAR STUDS/H.A.S. ATTACH TO CTR OF BM FLG SIZE OF STRUCT MEMBER
c. CAMBER MEASURED AT MIDSPAN ORDINATE. CAMBER TOLERANCE IS -0"/+1/2" OF INDICATED VALUE FOR SPAN UP TO 50' AND ADDL +1/8" PER 10' FOR SPAN OVER 50'
d. EL OF THE STRUCT MEMBER
e. DIFFERENCE IN EL OF STRUCT MEMBER FROM THE INDICATED TYP EL ON PLAN

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NUMBER PE-28957
PROFESSIONAL ENGINEER
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SAPP BUILDING RELOCATION TO BOONE COUNTY FAIRGROUNDS

Drawn:	Project Number:
BRH	08052.01
Checked:	CAD File Name (Number):
DCW	FAIR-S001.DWGL

Drawing Title: NOTES AND SCHEDULE

No.	Revisions:	Date:

Submission Date: 07-15-08
Plot Date: 07/11/2008

Drawing Number: S001
Sheet 1 of 3